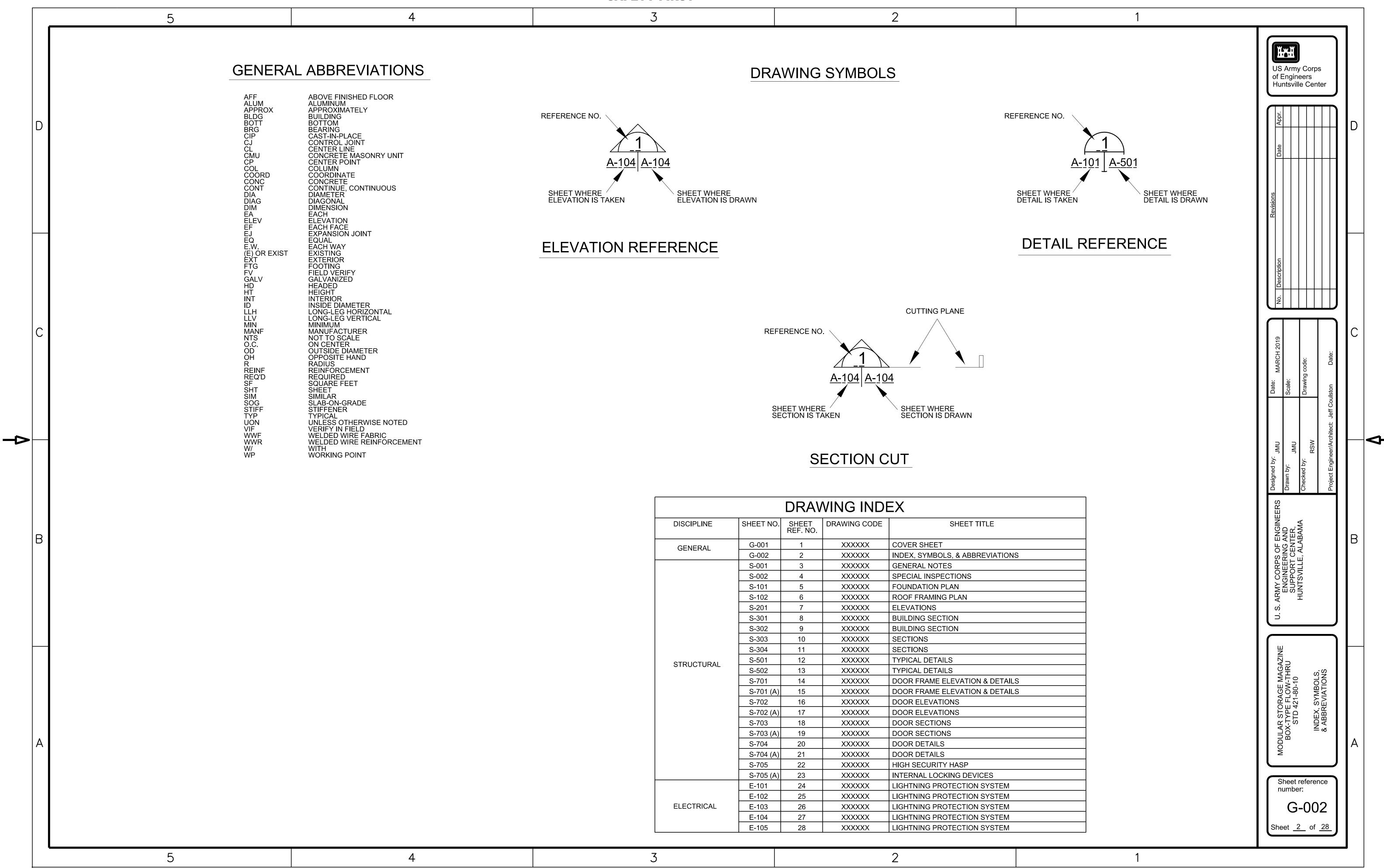
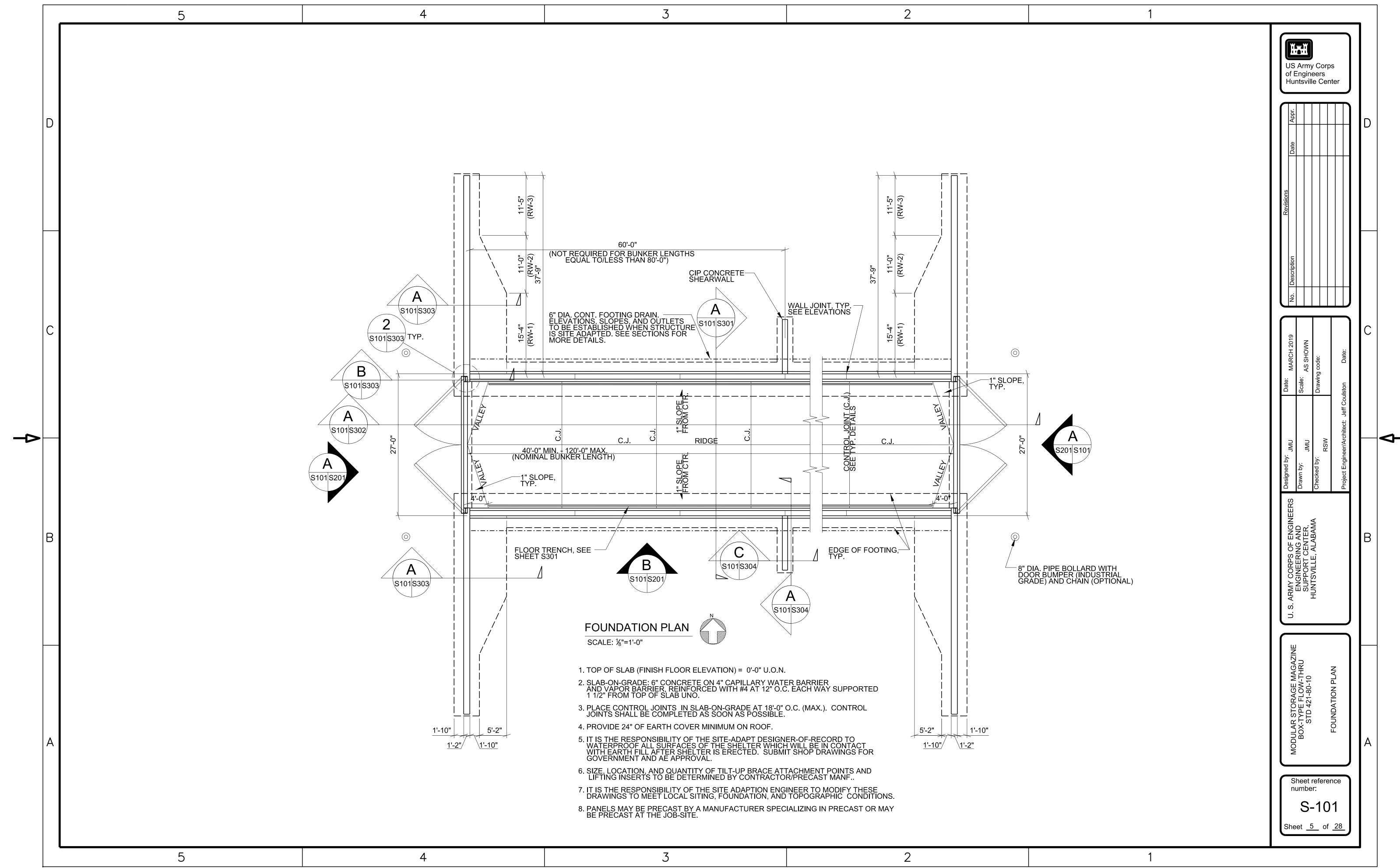
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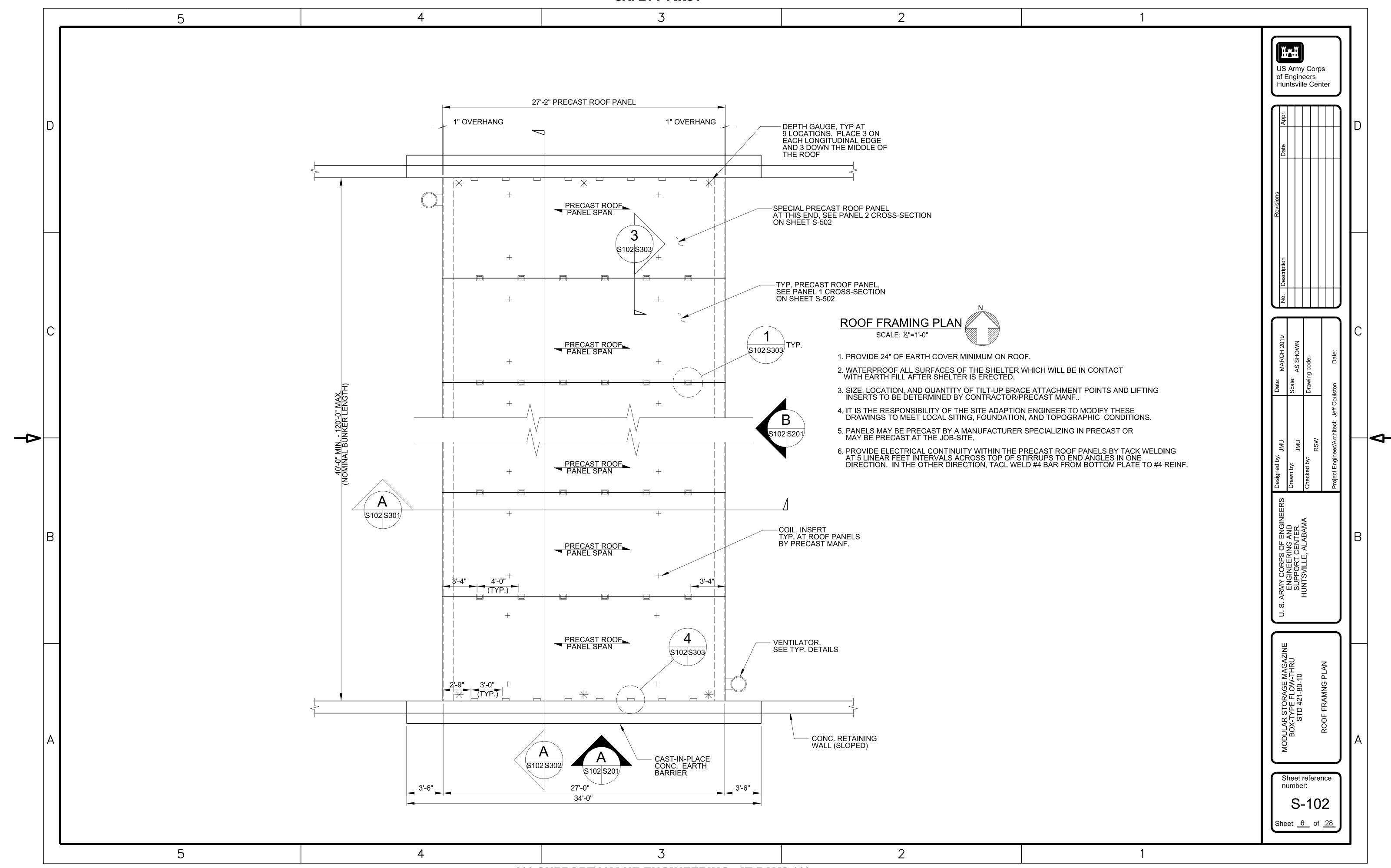
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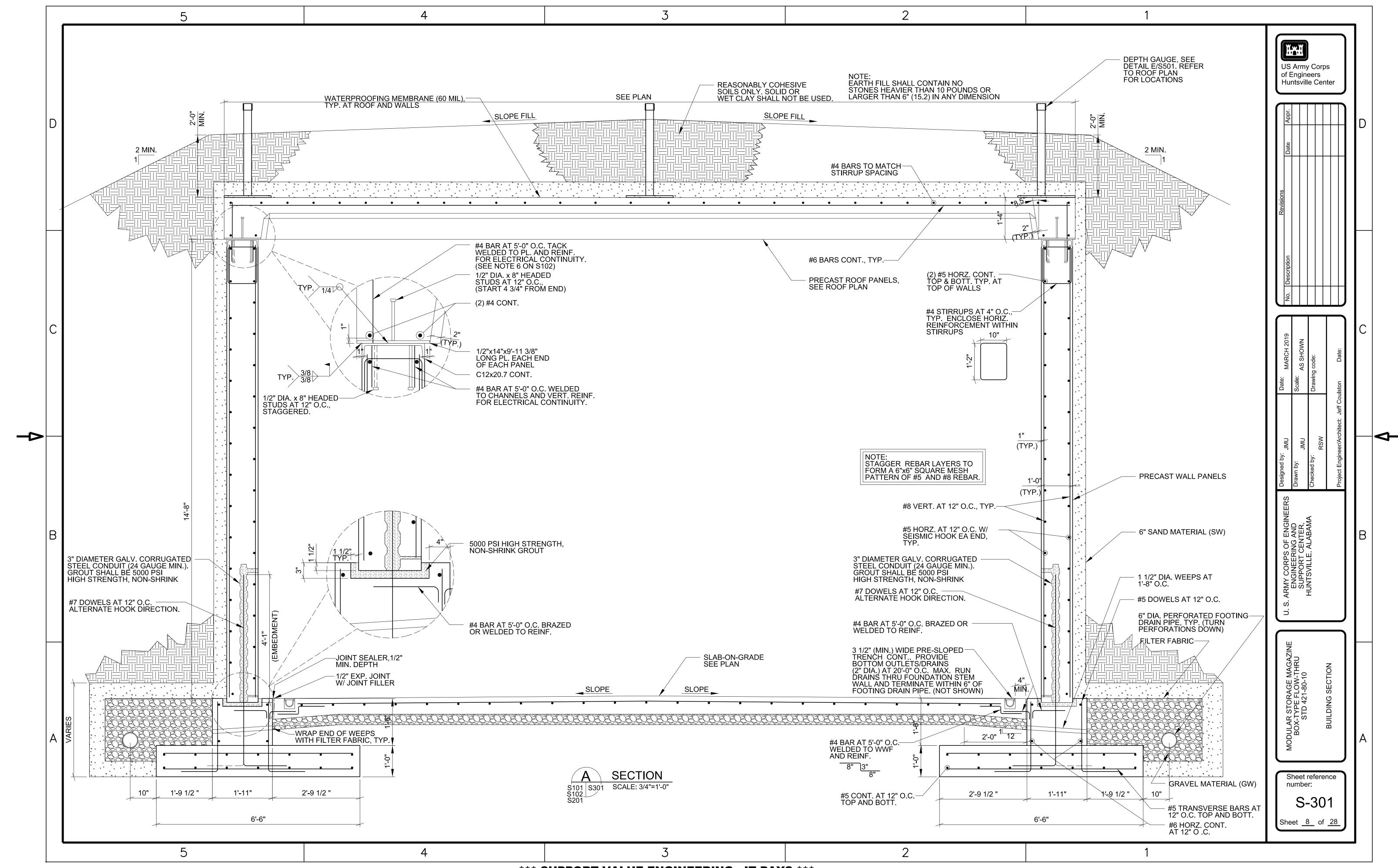


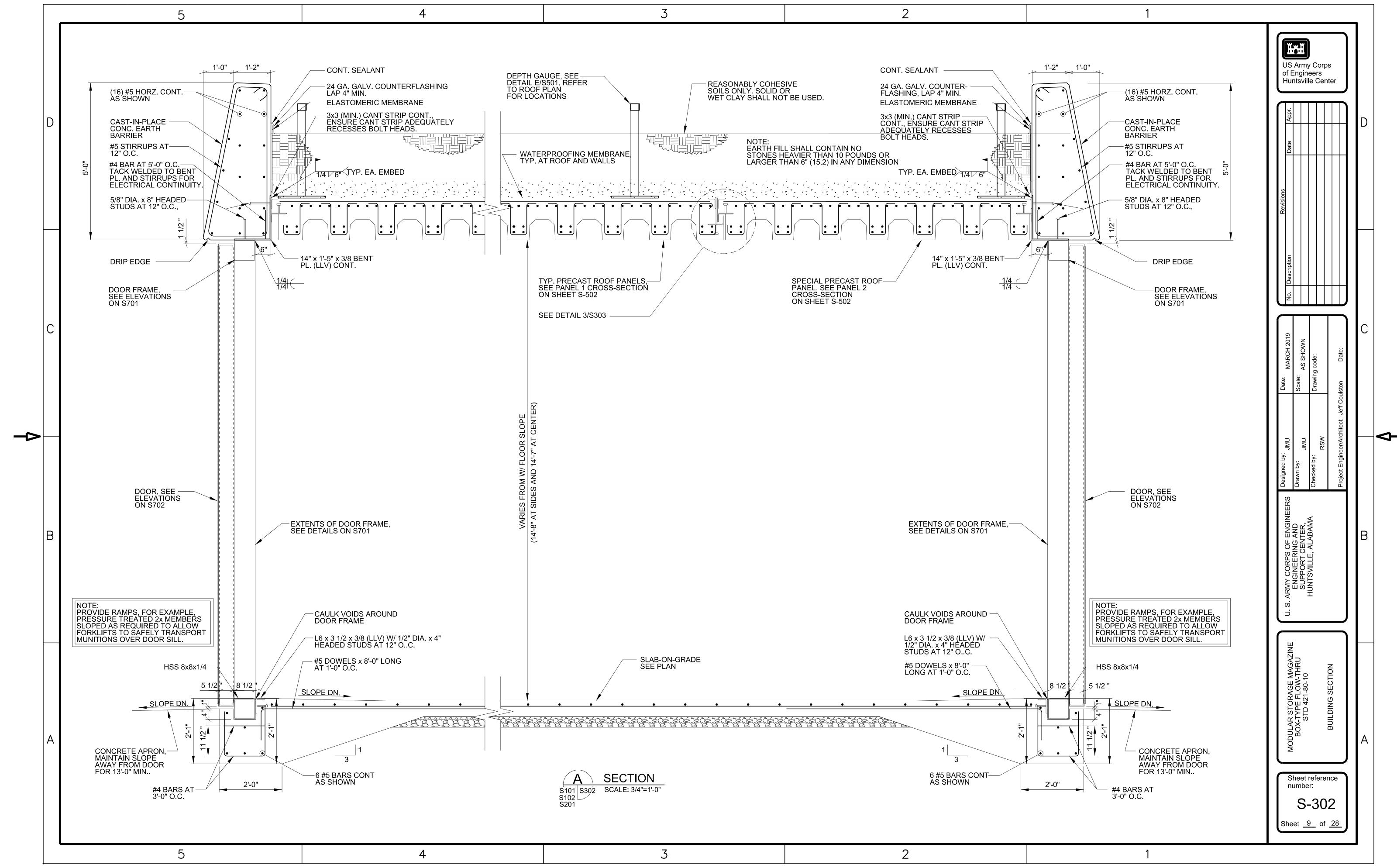
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		3.0 FOUNDATIONS		5.3 STRUCTURAL FASTENERS SHALL	CONFORM TO THE FOLLOWING ASTM DESIGNATIONS:			
	1.0 DESIGN CRITERIA:  A. BUILDING CODES AND SPECIFICATIONS:	3.1 SEE CIVIL DRAWINGS AND SPECIFICATIONS (PART OF SITE ADAPTION) FOR EARTHWORK PREPARATION OF FOUNDATIONS INCLUDING THE REMOVAL OF ORGANIC MATERIALS, COMPACTING SOILS BENEATH STRUCTURES, BACK FILL REQUIREMENTS FOR OVER EXCAVATION AND REMOVAL OF UNSUITABLE MATERIALS.		ANCHOR BOLTS THREADED RODS HEADED STUDS DEFORMED BAR ANCHORS	THREADED RODSASTM A36 HEADED STUDSASTM A108, GRADES 1015 TO 1020 (65 KSI TENSILE STRENGTH)			
	1. INTERNATIONAL BUILDING CODE 2012 (IBC) AS MODIFIED BY UFC 1-200-01 2. AMERICAN CONCRETE INSTITUTE (ACI 318) 3. AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC 13th ED.)		DWABLE NET SOIL BEARING PRESSURE USED FOR DESIGN: 3000 PSF.		ONFORM TO RCSC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING BOLTS SHALL BE 3/4" DIAMETER UNLESS OTHERWISE NOTED.	Tiditisvi	ville Center	
D	4. AMERICAN WELDING SOCIETY, A.W.S.  B. LIVE LOADS  ROOF100 PSF	3.4 ALL FOUNDATION BEARIN	OF SOIL USED FOR DESIGN: 120 PCF  IG SURFACES SHALL BE REVIEWED BY THE GEOTECHNICAL  CING CONCRETE TO ENSURE THEIR COMPLIANCE WITH THE		ONFORM TO AWS D1.1 "STRUCTURAL WELDING CODE-STEEL". MINIMUM 6" UNLESS OTHERWISE NOTED AND ELECTRODES SHALL BE E70xx. IN ACCORDANCE WITH AWS.	Appr.		
	FLOOR500 PSF SNOW LOAD:	3.5 ALL FOOTINGS SHALL PR	OJECT AT LEAST 1'-6" INTO UNDISTURBED NATURAL SOIL OR D FILL HAVING A SOIL BEARING PRESSURE THAT MEETS OR	SHALL BE DESIGNED BY A QUALIF STANDARD BEAM CONNECTIONS EQUAL TO ONE-HALF THE MAXIM	OON THE CONTRACT DRAWINGS, ALL FRAMED BEAM CONNECTIONS FIED PROFESSIONAL ENGINEER EMPLOYED BY THE FABRICATOR. (NON-COMPOSITE) SHALL BE DESIGNED BASED ON A REACTION UM TOTAL UNIFORM LOAD CAPACITY FROM AISC'S "MAXIMUM TOTAL	Date		
	GROUND SNOW LOAD (Pg) = 60 PSF IMPORTANCE FACTOR (I) = 1.1	3.6 ALL DISTURBED EARTH U	NDER FOOTINGS SHALL BE REPLACED WITH LEAN CONCRETE.		ED BY A FACTOR OF 1.2, UNLESS REACTIONS ARE SHOWN ON UM REACTION TO DESIGN FOR SHALL BE (12.0 KIPS).			ı
	EXPOSURE CATEGORY (Ce) = 1.0 THERMAL CATEGORY (Ct) = 1.2 C. WIND LOAD:	3.7 CONCRETE SHALL NOT B SUBJECTED TO WATER.	E PLACED OVER FROZEN SOIL OR FOOTING EXCAVATIONS	WITH A HIGH PERFORMANCE CO PROTECTION SHALL RECEIVE ON	TO THE WEATHER SHALL BE HOT DIPPED GALVANIZED OR COATED PATING SYSTEM (HPCS). MEMBERS NOT REQUIRED FOR CORROSION NE COAT OF STANDARD PRIMER PAINT. DO NOT PRIME OR PAINT	Revisions		
	BASIC WIND SPEED: 160 MPH	4.0 CONCRETE		CONCRETE COVER FOR ALL STE	EIVE FIELD WELDED HEADED SHEAR STUDS. PROVIDE 3" MINIMUM EEL BELOW GRADE AND PAINT WITH 2 COATS OF COAL TAR EPOXY. REMENTS OF PAINT SPECIFICATION SSPC-PAINT 16.			ı
	IMPORTANCE FACTOR (I): 1.0 EXPOSURE CATEGORY: C ENCLOSURE CLASSIFICATION: ENCLOSED	PLACING, FINISHING, AND	CLUDING DETAILING, FABRICATION, PLACEMENT OF REINFORCING, MIXING, HANDL CURING SHALL CONFORM TO THE FOLLOWING DOCUMENTS:	LING, 5.8 ALL STEEL EMBED PLATES, ANGL PANELS AND/OR CAST-IN-PLACE	LES, AND CHANNELS THAT ARE CAST INTO PRECAST CONCRETE CONCRETE, SLAB OR EARTH BARRIER/HEADER SHALL BE HOT-DIPPED IIZED MEMBERS SHALL BE TOUCHED UP IN ACCORDANCE WITH	iption		
	D. EARTHQUAKE:  RISK CATEGORY=III le= 1.25	ACI 315"MAN	NDARD SPECIFICATIONS FOR STRUCTURAL CONCRETE" UAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTUF DING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"	RES" ASTM A780.	PLATES SHALL BE MINIMUM 3/8" THICK, UNLESS OTHERWISE NOTED.	No. Descr		
	Ss= 0.65 Sds= 0.49 S1 = 0.18	4.2 ALL CONCRETE SHALL HA ALL CONCRETE SHALL CO	AVE A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS, U.O.N. ONFORM TO ASTM C94.	6.0 STRUCTURAL PRECAST CONCRE	TE		<del></del>	
C	St = 0.16 Sd1 = 0.19 SITE CLASS: C BASIC SEISMIC-FORCE RESISTING SYSTEM=		LL BE DEFORMED TYPE CONFORMING TO ASTM A615 GRADE 60 U.O.N. CEMENT SHALL CONFORM TO ASTM A185. MINIMUM LAP AND EMBEDMENT TO BE TI	THE CONCRETE AND CONSTRUCTION	ETAILED ON DRAWINGS SHALL BE DESIGNED FOR THE SPAN AND LOADING CONDITIONS SHOWN ON THE DRAWINGS BY A LICENSED	019		
	INTERMEDIATE PRECAST SHEAR WALLS, R = 4 SEISMIC DESIGN CATEGORY= C ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE PROCEDURE	GREATER OF ONE CROSS	EMENT SHALL CONFORM TO ASTM A 165. MINIMUM LAP AND EMBEDMENT TO BE	STRUCTURAL ENGINEER. ALL DE ELEMENTS AND LIFTING POINTS	ESIGN CALCULATIONS, INCLUDING THE DESIGN OF ALL STRUCTURAL SHALL BE SUBMITTED TO THE CONTRACTING OFFICER FOR REVIEW	IARCH 2	ode: Date:	
	E. SOILS		PECIFICATIONS. REINFORCING SHALL NOT BE WELDED IN ANY MANNER U.O.N. IN		SHALL BE RESPONSIBLE FOR COORDINATION OF ALL DISCIPLINES AS	Date: M Scale:	Drawing co	
	SOIL DENSITY (ɣ): 120 PCF ANGLE OF INTERNAL FRICTION OF THE SOIL (Φ) : 30 DEGREES EQUIVALENT FLUID PRESSURE (EFP) : 60 PSF PER FOOT OF DEPTH (AT-REST)		CONTINUOUS WITH CLASS "B" TENSION LAP SPLICES, U.O.N.  OF REINFORCEMENT FOR CAST-IN-PLACE CONSTRUCTION U.O.N.:	6.3 THERE SHALL BE NO FIELD CUTT CONTRACTING OFFICER.	ING OF PRECAST ELEMENTS WITHOUT THE APPROVAL OF THE		Teff Couls	
	2.0 GENERAL	CONCRETE CAST AGAI	NST EARTH:3 INCHES	6.4 CONCRETE SHALL HAVE A MINIM	UM COMPRESSIVE STRENGTH AT TWENTY-EIGHT DAYS OF 4000 PSI.		chitect: ,	ı
	2.1 CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS PRIOR TO CONSTRUCTION/FABRICATION. CONTRACTOR SHALL NOTIFY CONTRACTING OFFICER OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION.	NO. 6 BAR AND LA NO. 5 BAR AND SI CONCRETE NOT EXPOS	MALLER1 1/2 INCHES SED TO WEATHER:		NK, NON-METALLIC WITH F'c = 6000 PSI.	d by: JMU y: JMU	I by: RSW :ngineer/Ard	
	2.2 THE STRUCTURE (MEMBERS AND CONNECTIONS) HAS BEEN DESIGNED TO SUPPORT IN-PLACE DESIGN LOADS ONLY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR LIMITING	SLABS, WALLS, JO BEAMS AND COLO SLAB ON GRADE.	UMNS1 1/2 INCHES		REINFORCEMENT IN FLOOR, PRECAST WALLS AND ROOF PANELS, S AND DOOR FRAME. SHALL BE MADE ELECTRICALLY CONTINUOUS	Designe Drawn b	Checked Project E	
	CONSTRUCTION LOADS SUCH THAT THESE LOADS DO NOT EXCEED THE DESIGN LOADS NOTED ABOVE.  2.3 IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE CONSTRUCTION METHODS,		BARS IN CONCRETE FOOTINGS TO MATCH THE SIZE AND SPACING OF THE HORIZON RNERS AND INTERSECTIONS OF STRIP FOOTINGS. PROVIDE LEG LENGTH EQUIVAL AP SPLICE U.O.N.	NTAL BY BONDING (CLIPPING, BRAZING CONTINUITY SHALL BE PROVIDED PEDESTALS AND PRECAST ROOF	OR TACK WELDING) AT 5 LINEAR FEET INTERVALS. ELECTRICAL DACROSS FLOOR EXPANSION AND ISOLATION JOINTS TO FOUNDATION F PANELS, AND BETWEEN PRECAST WALLS AND CONCRETE PEDESTAL URING CONSTRUCTION. ACCEPTABLE CONTINUITY METHODS ARE	NEERS	⋖	
В	PROCEDURES, AND SEQUENCES TO ENSURE STABILITY AND SAFETY DURING CONSTRUCTION. THE CONTRACTOR SHALL TAKE NECESSARY PRECAUTIONS TO PROTECT AND MAINTAIN THE STRUCTURAL INTEGRITY OF ALL NEW AND EXISTING CONSTRUCTION AT ALL STAGES.		NDATION WITH 90 DEGREE HOOK TO MATCH SIZE AND SPACING OF VERTICAL DESTALS, WALLS, AND COLUMNS.	REINFORCING BARS (MINIMUM O	VERLAP SHALL BE 20 BAR DIAMETERS), COPPER STRAPS, ETC. R ADDITIONAL INFORMATION REGARDING LPS.	OF ENGI	LABAM	
	2.4 SECTIONS AND DETAILS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE CONSIDERED TYPICAL FOR SIMILAR CONDITIONS THAT DO NOT HAVE A SPECIFIC SECTION INDICATED.		SHALL HAVE NO HORIZONTAL JOINTS (POURED TO THEIR FULL DEPTHS IN STOP IN CONCRETE WORK SHALL BE BULKHEAD AND KEYED, U.O.N.	DESIGNER NOTES: TO BE REMOVED FOR SITE ADAPTION OF THIS DES	/ED WHEN PREPARING CONSTRUCTION DRAWINGS	ORPS C NEERIN ORT CE	ULLE, A	
	2.5 THE CONTRACTOR SHALL COORDINATE STANDARD DRAWINGS WITH THE VENDOR/MANF. SHOP DRAWINGS TO VERIFY SIZES AND LOCATIONS OF OPENINGS, SLEEVES, INSERTS, DEPRESSIONS, FINISHES, SLOPES, ETC. ANY DISCREPANCY SHALL BE BROUGHT TO		NOT BE BENT OR STRAIGHTENED IN A MANNER THAT WILL DAMAGE THE WITH KINKS OR IMPROPER BENDS SHALL NOT BE USED.	TO BE ADEQUATE UNDER THES		RMY COENGIN	HUNTS	
	THE ATTENTION OF THE CONTRACTING OFFICER.  2.6 SEE CIVIL SITE LAYOUT DRAWINGS (PART OF SITE ADAPTION) FOR ACTUAL FINISHED FLOOR	4.12 REINFORCEMENT SHALL THROUGH ALL CONTROI	BE CONTINUOUS THROUGH ALL CONSTRUCTION JOINTS, BUT DISCONTINUOUS L JOINTS, U.O.N		ECIFIC LOADING CRITERIA. IF SITE-SPECIFIC LOADS EXCEED , THE DESIGNER SHOULD ADDRESS ALL DEFICIENCIES THAT	U. S. A	_	
	ELEVATIONS (F.F.E.) FOR ALL BUILDÌNGS. ELEVATIONS SHOWN IN STRUCTURAL DOCUMENTS WILL BE BASED ON REFERENCED F.F.E. EQUAL TO 100'-0", U.O.N.		QUIRED FOR EXPOSED FORMED SURFACES OF PRECAST PANELS. A CLASS D R SURFACES WHICH WILL BE BELOW GRADE OR NOT EXPOSED TO VIEW '.		SED TO REFLECT SPECIFIC SITE SOIL CONDITIONS INCLUDING CONDITIONS, AND FROST PENETRATION DEPTHS.		$\overline{}$	
	2.7 ANY DISCREPANCIES BETWEEN DRAWINGS, SPECIFICATIONS, REFERENCE STANDARDS, OR GOVERNING CODE, THE MORE STRINGENT REQUIREMENTS SHALL GOVERN. CONTRACTOR SHALL NOTIFY THE CONTRACTING OFFICER OF DISCREPANCIES AND OBTAIN DIRECTION PRIOR TO PROCEEDING.	4.13 REFER TO GEOTECHNIC FOR SLAB ON GRADE W	AL REPORT FOR RECOMMENDATIONS RELATIVE TO SUBGRADE PREPARATION ORK.	SLAB-ON-GRADE, AND WING W THE CONTRACTING OFFICER, V	WITH THE EXCEPTION OF THE FOUNDATION (FOOTINGS),  YALLS SHALL NOT BE MODIFIED WITHOUT THE APPROVAL OF WHO SHOULD CONSULT WITH THE U.S. ARMY ENGINEERING SVILLE (STRUCTURAL BRANCH). STRUCTURE HAS BEEN	MAGAZIN THRU	10 ES	
	2.8 CONTRACTOR SHALL PROVIDE TEMPORARY SHORING AND BRACING OF ALL STRUCTURAL WORK, AND SOIL EXCAVATION AS REQUIRED. SHORING AND BRACING SHALL NOT BE	5.0 STRUCTURAL STEEL		DETERMINED TO BE ADEQUATI	E FOR THE DESIGN CRITERIA LISTED ON THIS SHEET.  CURITY HASP) AND S701(A) - S705(A) (ILD) IDENTIFY TWO	RAGE FLOW-21-80-1	21-80-	
	REMOVED UNTIL ALL FINAL CONNECTIONS HAVE BEEN COMPLETED IN ACCORDANCE WITH THE DRAWINGS, AND MATERIALS HAVE ACHIEVED DESIGN STRENGTH.	A.I.S.C.'S "SPECIFICATION	RICATION, ERECTION, AND CONNECTION DESIGN SHALL CONFORM TO I FOR STRUCTURAL STEEL BUILDINGS."	DIFFERENT LOCKING SYSTEMS OFFICER THE CORRECT LOCKI	S. THE DESIGNER SHALL VERIFY WITH THE CONTRACTING ING SYSTEM REQUIRED AND REMOVE THE REDUNDANT CTION CONTRACT DOCUMENTS FOR THE SYSTEM NOT USED.	AR STO	STD 4	
A		W SHAPES	LL CONFORM TO THE FOLLOWING ASTM DESIGNATIONS:ASTM A36			BO)	J	
		STEEL CHANNELS, AND (MAXIMUM YIELD RECTANGULAR, SQUAF	GLES, PLATES AND BARS:ASTM A36 STRESS (Fy) SHALL NOT EXCEED 50 KSI FOR DOOR CONSTRUCTION) RE, AND ROUND HSSASTM A500, GRADE B		NDARD APPROVED DESIGN DRAWINGS FOR THE CONCRETE	\ \		
		STEEL PIPE (HSS)	BE FABRICATED FROM DUAL GRADED ASTM A36 AND ASTM A572 GRADE 50	APPROVAL FROM THE DEPARTI MAY REQUIRE THE MAGAZINE T	CRETE ROOF OR THEIR SUPPORTS WITHOUT WRITTEN MENT OF DEFENSE EXPLOSIVE SAFETY BOARD (DDESB) TO BE CONSIDERED AN UNDEFINED MAGAZINE AND MAY	Sheet number	et reference ber:	ĺ
		STRUCTURAL STEEL PLATEDOORS SHALL BE LE	TES. THE YIELD STRESS (fy) OF ALL STUCTURAL STEEL PLATES USED IN SS THAN 50 KSI. TO ENSURE SATISFACTION OF THIS REQUIREMENT, THE L. SUBMIT CERTIFIED MANUFACTURER'S MILL REPORTS FOR ALL STRUCTURAL		DRAWINGS, FACILITY MEETS BLAST-RESISTANT DESIGN	s	S-001	l
		STEEL PLATES USED IN T		CRITERIA FOR A 7-BAR STRUCT	TURAL DESIGNATION PER DESR 6055.09, EDITION 1. THIS ES VALIDATION OF THE DESIGN AGAINST OTHER LOAD CASES.	Sheet _	<u>3</u> of <u>28</u>	
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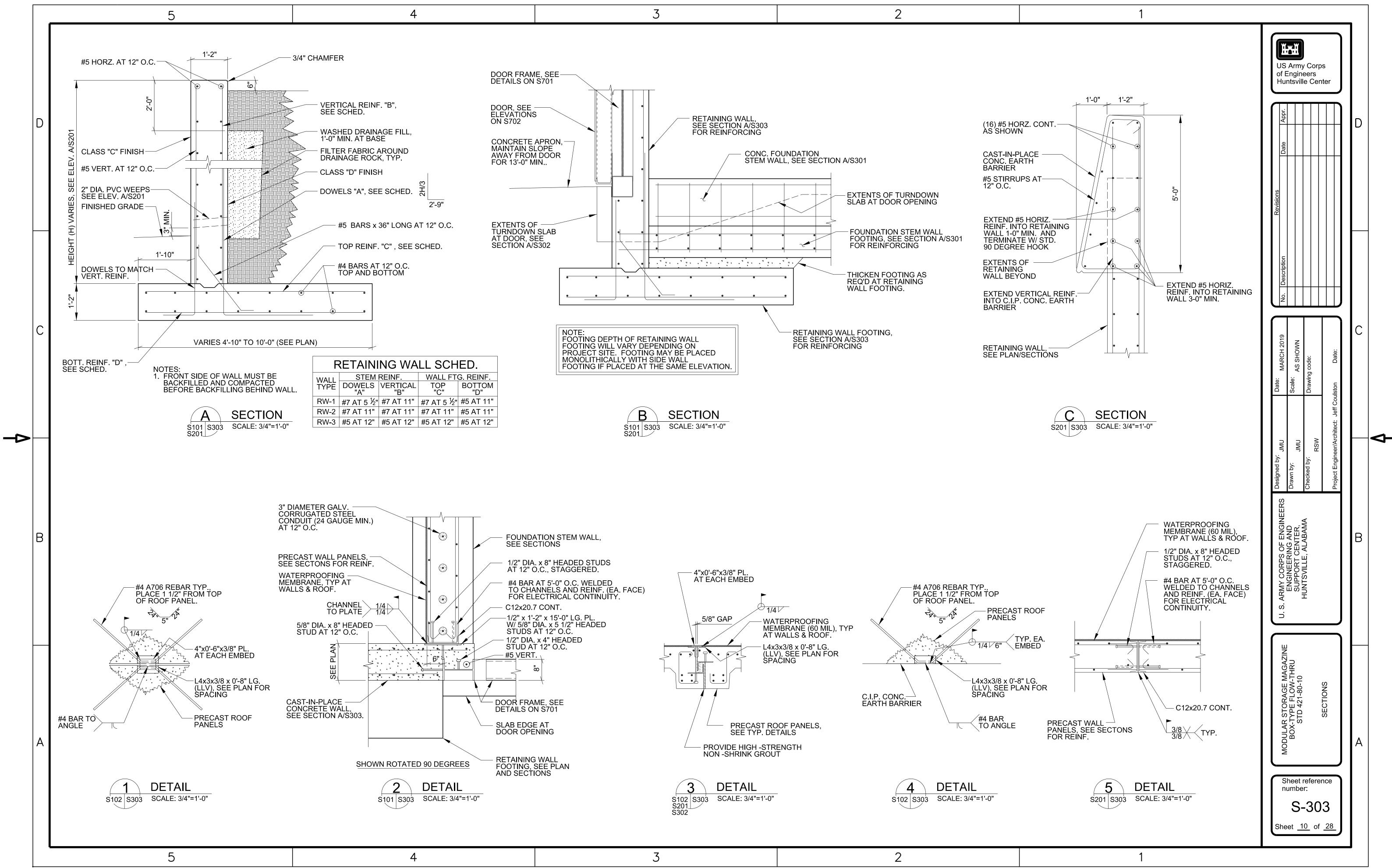
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	SPEC	CIAL INSPE	CTION SCHEDULE/VERIFICATION		of Engi Huntsv
ITEM	EXTENT OF	REFERENCE	COMMENTS/SCOPE		
CONCRETE CONSTRUCTION	INSPECTION 1				4ppr.
REINFORCING STEEL PLACEMENT	P AC	CI 318: 3.5, 7.1-7.7	INSPECT SIZE, SPACING, COVER, POSITIONING AND GRADE OF REINFORCING STEEL. VERIFY THAT REINFORCING BARS ARE FREE OF FORM OIL OR OTHER DELETERIOUS MATERIALS. INSPECT BAR LAPS AND MECHANICAL SPLICES. VERIFY THAT BARS ARE ADEQUATELLY TIED AND SUPPORTED ON CHAIRS OR BOLSTERS	DESIGNER NOTES: TO BE REMOVED WHEN PREPARING CONS FOR SITE ADAPTION OF THIS DESIGN.  1. SPECIAL INSPECTION SCHEDULE SHALL BE REVISED TO RE	
WELDING OF RIENFORCEMENT	C, P		VISUALLY INSPECT ALL REINFORCING STEEL WELDS. VERIFY WELDABILITY OF REINFORCING STEEL. INSPECT PREHEATING OF STEEL WHEN REQUIRED.	REQUIREMENTS IN ACCORDANCE WITH CHAPTER 17 OF THI HOWEVER, AT A MINIMUM THE SPECIAL ITEMS RELATED TO	E INTERNATIONAL BUILDING CODE;
CONCRETE PLACEMENT	C A	ACI 318: 5.9, 5.10	INSPECT PLACEMENT OF CONCRETE. VERIFY THAT CONCRETE CONVEYANCE AND DEPOSITING AVOIDS SEGREGATION OR CONTAMINATION. VERIFY THAT CONCRETE IS PROPERLY CONSOLIDATED	RELATED ITEMS' SHALL BE INSPECTED AS SHOWN ON THIS	
SAMPLING AND TESTING OF CONCRETE	С	ASTM C 172 ASTM C 31	TEST CONCRETE COMPRESSIVE STRENGTH, SLUMP, AIR-CONTENT AND TEMPERATURE		ous
CURING AND PROTECTION		ACI 318: 5.6, 5.8	INSPECT CURING, COLD WEATHER PROTECTION AND HOT WEATHER PROTECTION PROCEDURES		Revisi
FORMWORK			INSPECT FORWORK FOR SHAPE, LOCATION, AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED		
PRECAST CONCRETE					
PLANT CERTIFICATION/QUALITY CONTROL PROCEDURES	S		REVIEW OF PLANT OPERATIONS AND QUALITY CONTROL PROCEDURES		
MIX DESIGN MATERICAL CERTIFICATION	S		INSPECT CONCRETE BATCHING OPERATIONS AND VERIFY COMPLIANCE WITH APPROVED MIX DESIGN REVIEW FOR CONFORMANCE TO ACI 318		escrip
REINFORCEMENT INSTALLATION  CONNECTIONS/EMBEDDED ITEMS	P		INSPECT SIZE, SPACING, POSITION AND GRADE OF REINFORCING STEEL INSPECT INTERFACE CONNECTIONS INCLUDING END AND EDGE DOWELING. INSPECT EMBEDMENTS FOR PROPER		
CONCRETE PLACEMENT	, , , , , ,		LOCATION AND WELDING OF CONNECTIONS INSPECT PLACEMENT OF CONCRETE. VERIFY THAT CONCRETE CONVEYANCE AND DEPOSITING AVOIDS		
SAMPLING AND TESTING	C	401310. 3.9, 3.10	SEGREGATION OR CONTAMINATION. VERIFY THAT CONCRETE IS PROPERLY CONSOLIDATED		
CURING AND PROTECTION	Р		INSPECT ERECTION OF PRECAST CONCRETE INCLUDING MEMBER CONFIGURATION, CONNECTIONS, WELDING AND		5019
ERECTED PRECAST ELEMENTS			GROUTING		Å Å CH
DOOR CONSTRUCTION  FABRICATOR CERTIFICATION/QUALITY CONTROL					
PROCEDURES	S		REVIEW OF FABRICATOR'S QUALITY CONTROL PROCEDURES OR AISC CERTIFICATION  INSPECT IN-PLANT FABRICATION, OR REVIEW FABRICATOR'S APPROVED INDEPENDENT INSPECTION AGENCY'S		Date
FABRICATOR INSPECTION	P		REPORTS		
SPECIAL ITEMS RELATED TO THE OTHER					
EXPLOSIVES SAFETY RELATED ITEMS	D 5		INSPECT REINFORCING STEEL TO ENSURE ELECTRICAL CONTINUITY BETWEEN THE CAP, WALLS, SLAB AND		
REBAR FARADAY-SHIELD		//GS E-101, E-102	FOUNDATION THROUGH BONDING WELDS. DOCUMENT BONDS WITH PHOTOS AND CONTINUITY TEST.		
ECM GROUNDNG		77 G3 L-101, L-102	PHOTOS.		igned l
GROUNDING SYSTEM	P DAF	NFPA 780, PAM 385-64, 17-27.	VISUALLY INSPECT GROUNDING SYSEM CONDUCTORS TO ENSURE NO DAMAGE, BREAKAGE, OR CORROSION HAS OCCURRED TO THE CONDUCTORS DURING INSTALL AND BEFORE EARTH BURIAL.		
INDIVIDUAL BONDS	P	VGS E-101, E-102, NFPA 780, 8.9 PAM 385-64, 17.27.	INSPECT ALL BONDS FOR LOOSE CONNECTIONS THAT MIGHT RESULT IN HIGH-RESISTANCE CONNECTIONS.		G NE ER
LPS COMPONENTS		NFPA 780, 8.9 PAM 385-64, 17-27.	INSPECT LPS COMPONENTS FOR SECURE MOUNTING AND PROTECTION AGAINST ACCIDENTAL MECHANICAL DISPLACEMENT.		OF EN THE STATE OF
LPS TESTING			PERFORM BONDING TEST ACROSS EACH BOND, AND AN EARTH ELECTRODE TEST OF THE LPS.		RMY CORPS OF ENGI ENGINEERING AND
EARTH COVER	P D		INSPECT DEPTH GAUGES ON ROOF PRIOR TO EARTH COVER PLACEMENT FOR SIZE AND STABILITY. INSPECT EARTH COVER DEPTH AND SLOPE TO ENSURE A 2' MIN. IS PROVIDED ABOVE STRUCTURE		
DOOR LAPS	С	DWG S-701	INSPECT DOOR LAPS AT TOP AND BOTTOM OF DOOR FRAME		S. AR
1 INSPECTION INTERVALS ARE AS FOLLOWS:	<b></b>		CIAL INSPECTION NOTES:		
<b>P</b> - Periodic: The part-time or intermittent observation o			ecial inspector who is present in the area where the work is being performed pproved special inspector who is present in the area where the work has been or is being performed and at the completion of the		
work. <b>S</b> - Submittal					N N N N N N N N N N N N N N N N N N N
2 STRUCTURAL TEST AND SPECIAL INSPECTIONS A 3 CONTRACTOR SHALL HIRE A QUALIFIED INSPECTI			C 2009 EDITION RM SPECIAL INSPECTIONS AND TESTING IN ACCORDANCE WITH THE IBC. SUBMIT INSPECTION REPORTS TO THE		IAGA.
CONTRACTING OFFICER FOR EACH DAY SPECIAL					GE N GG
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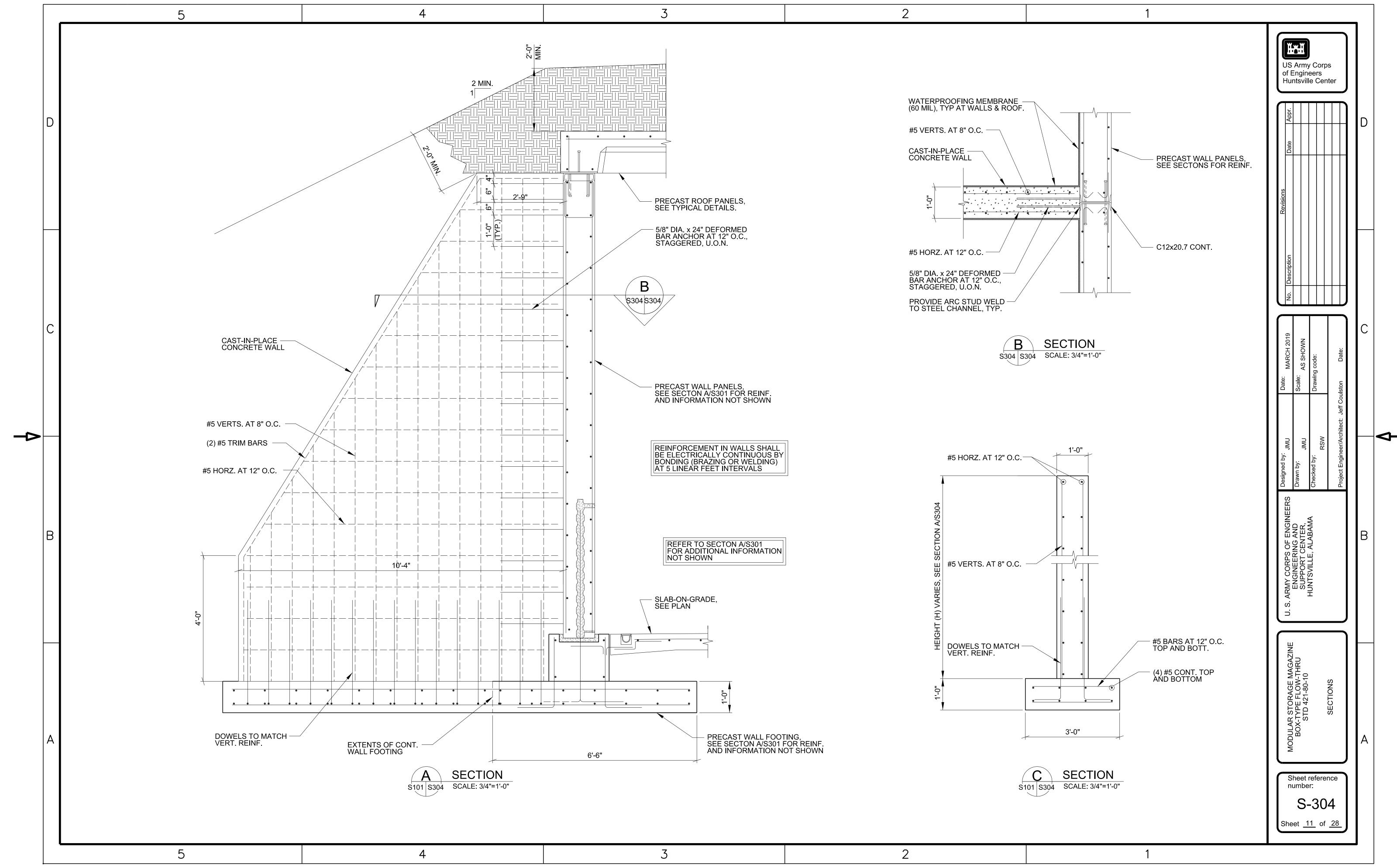


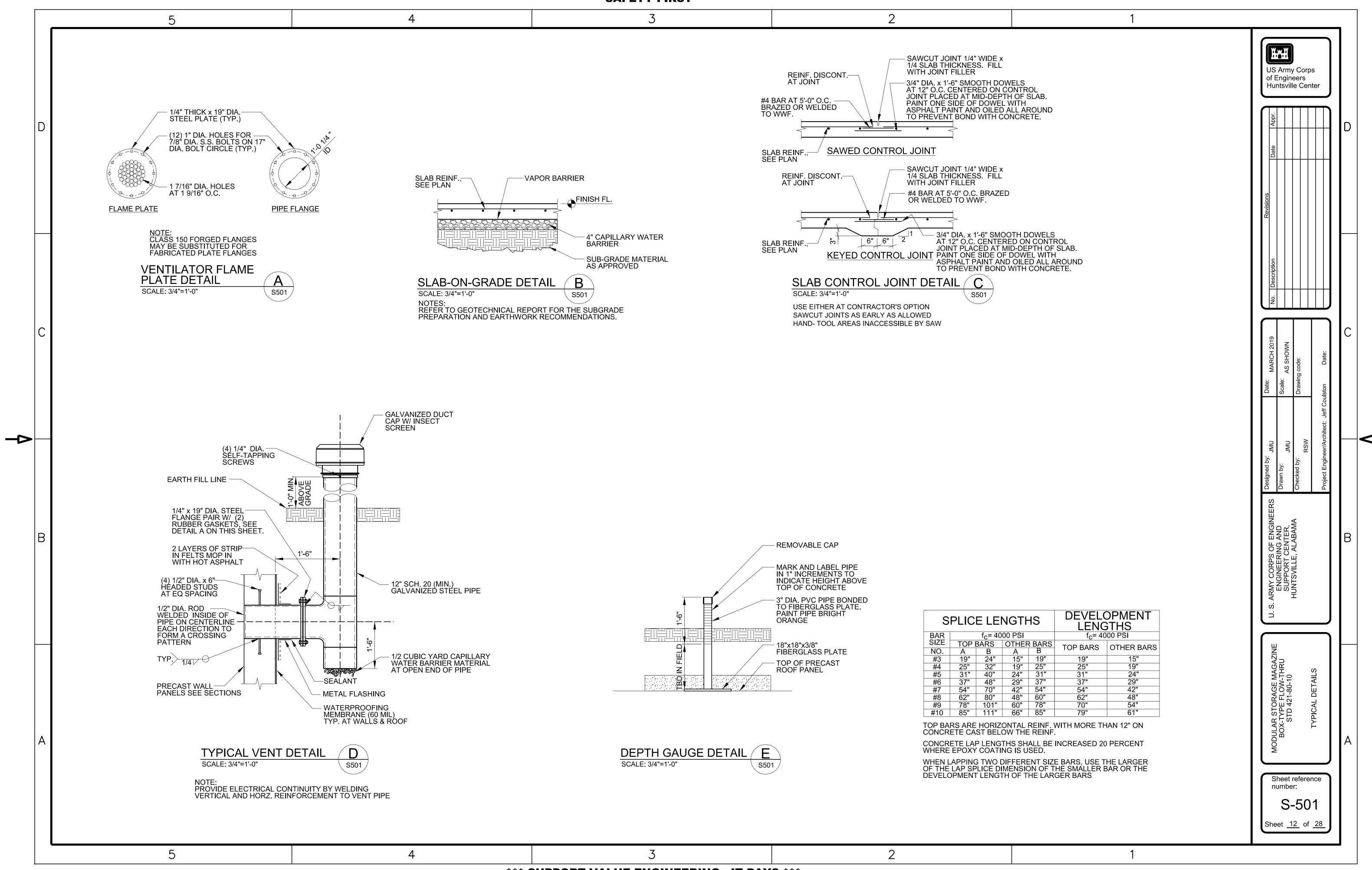


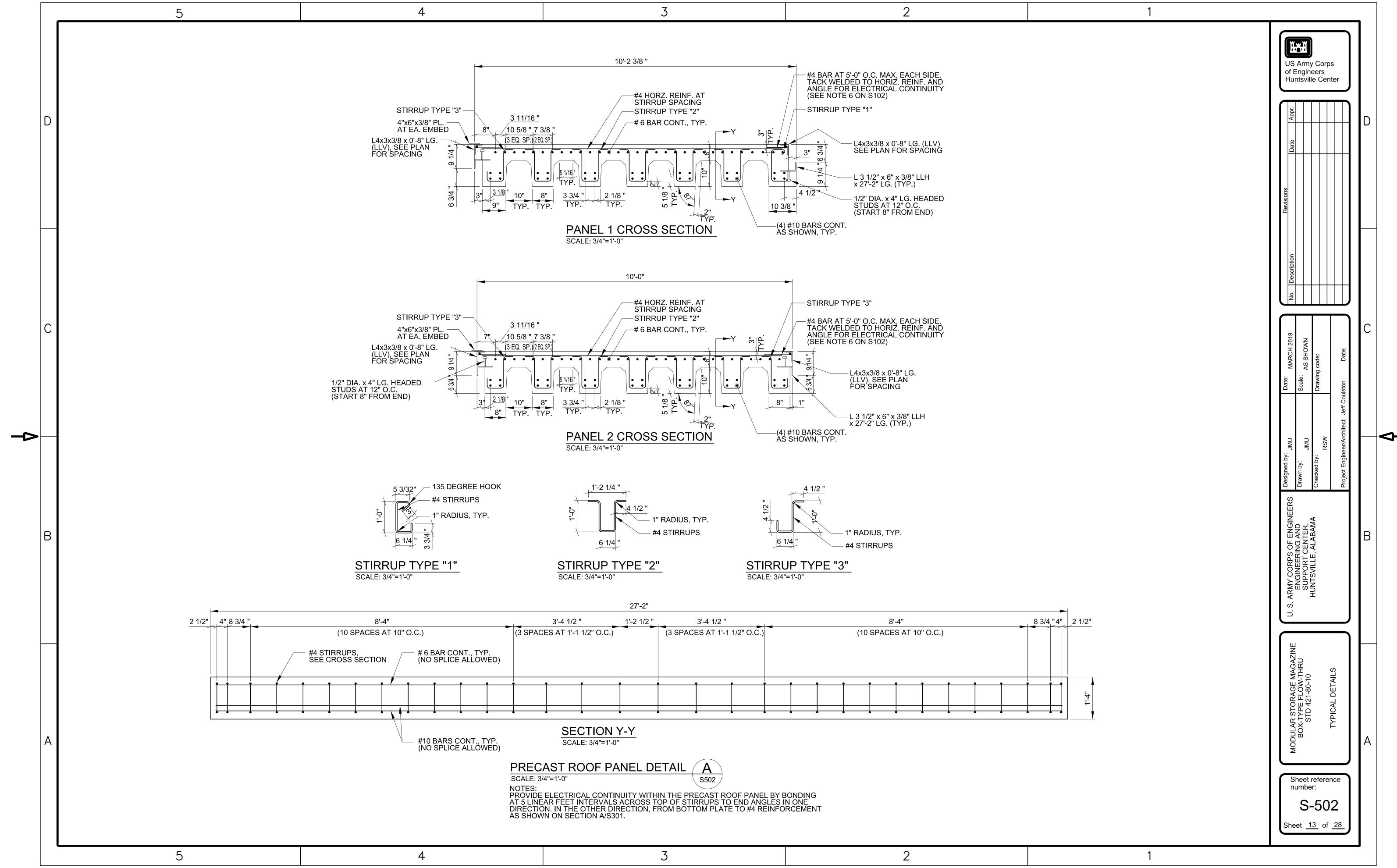


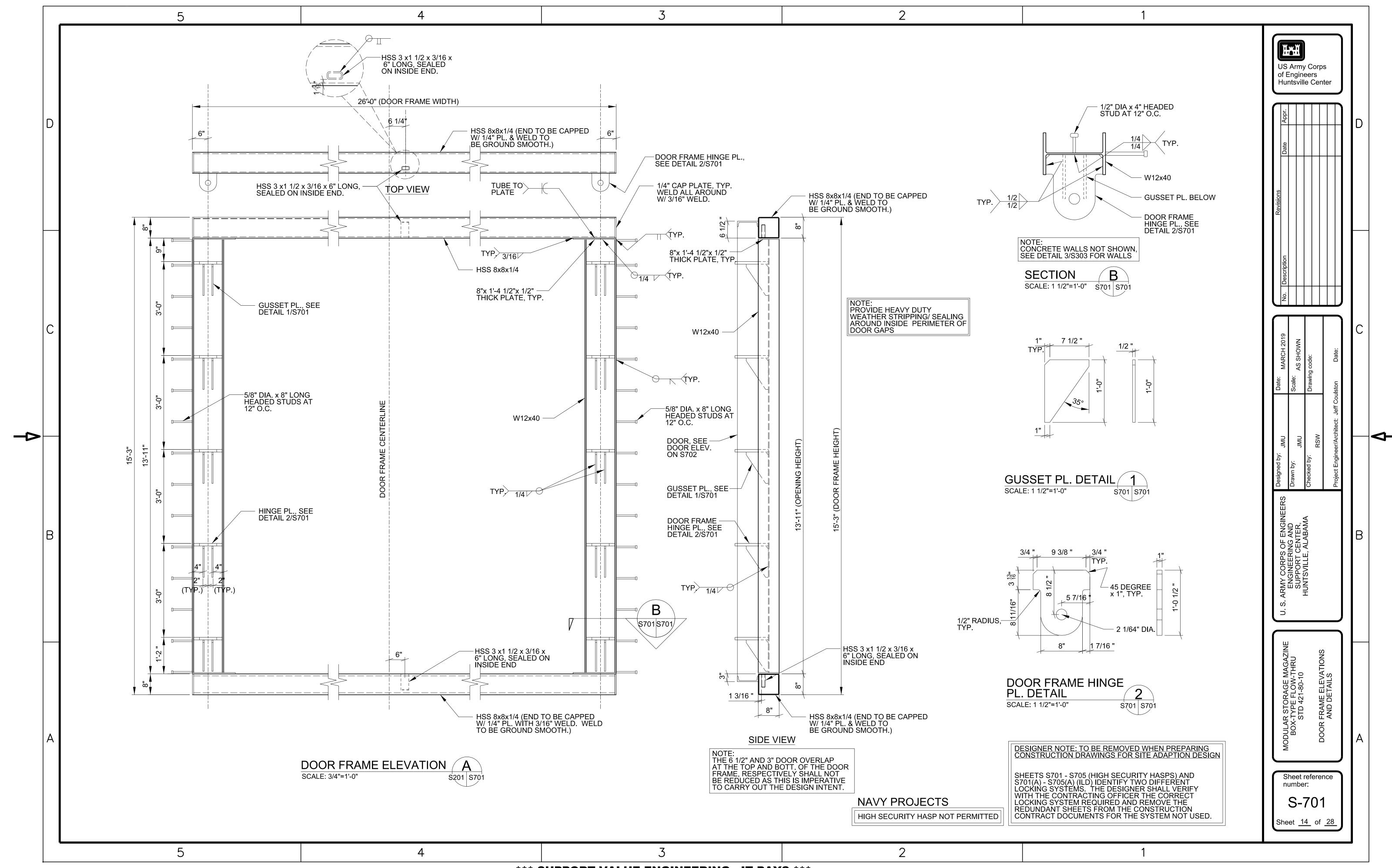


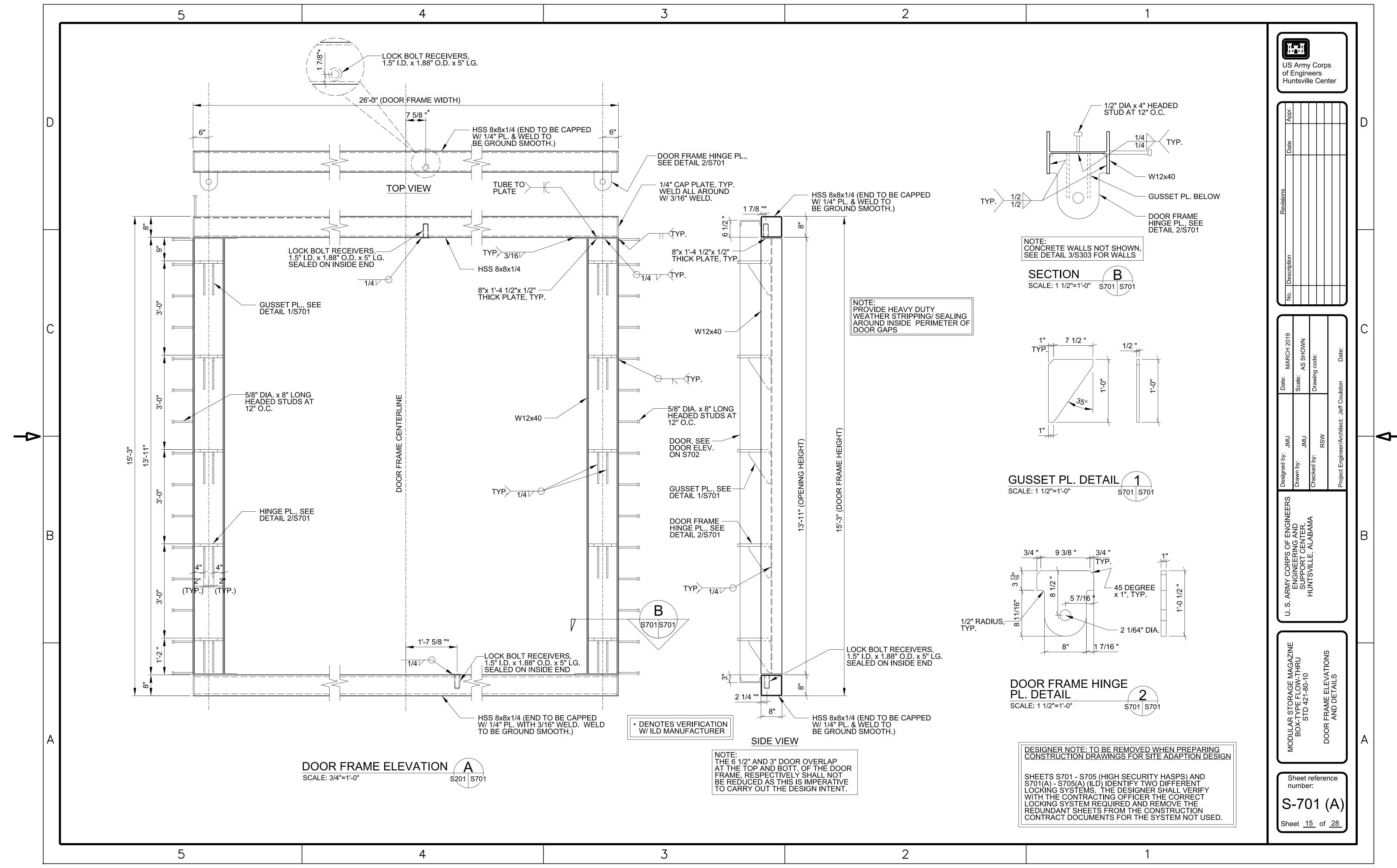




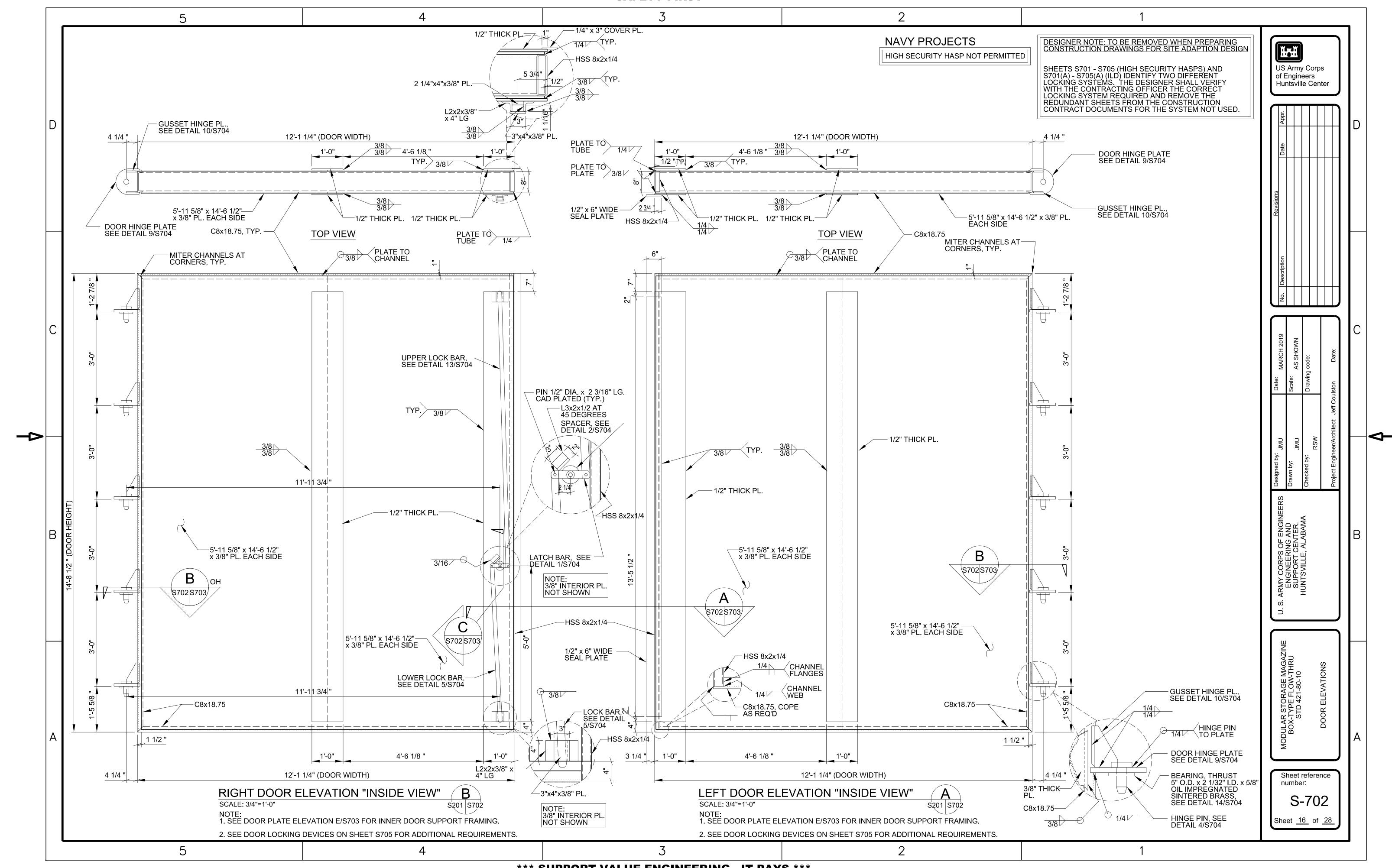




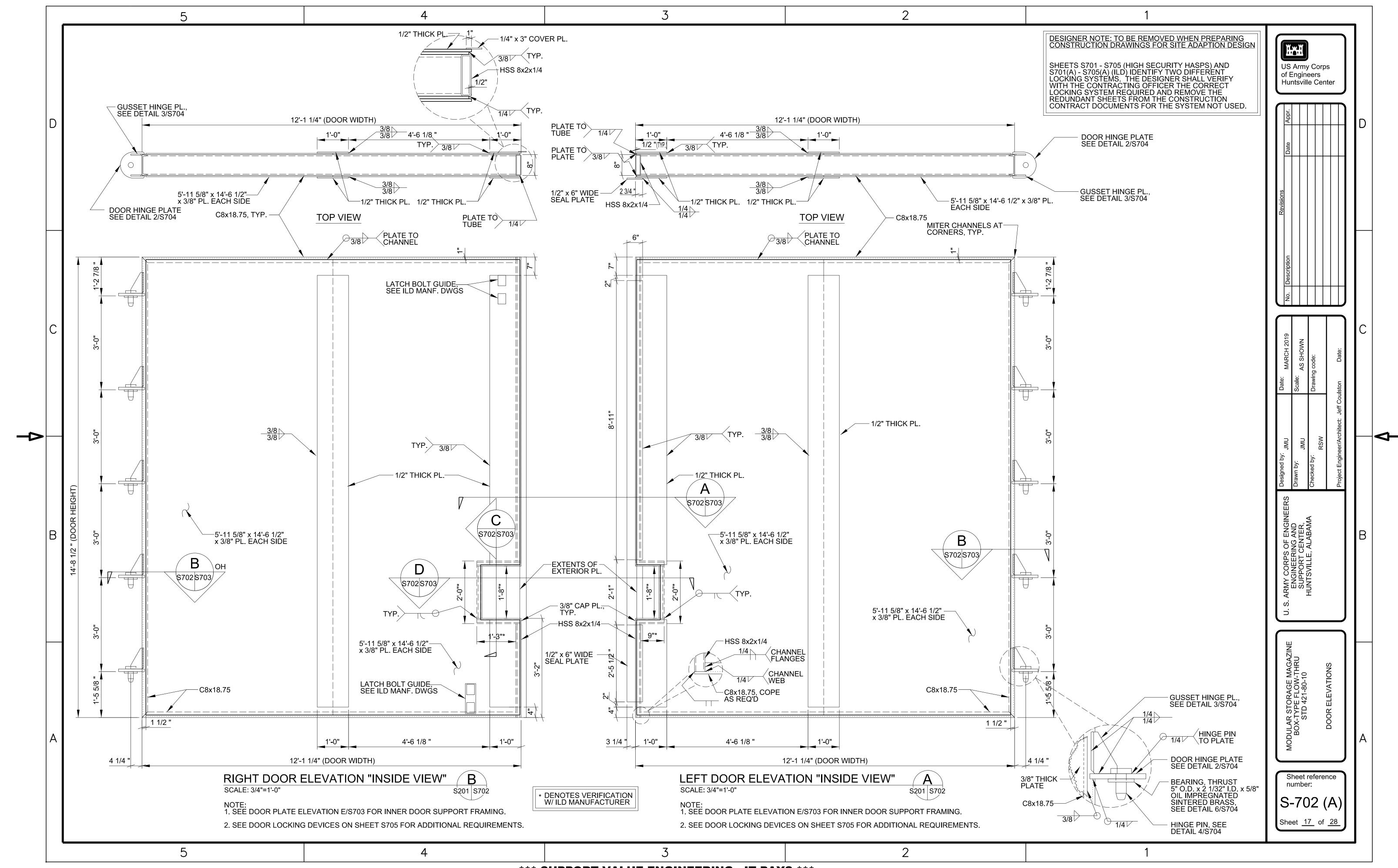


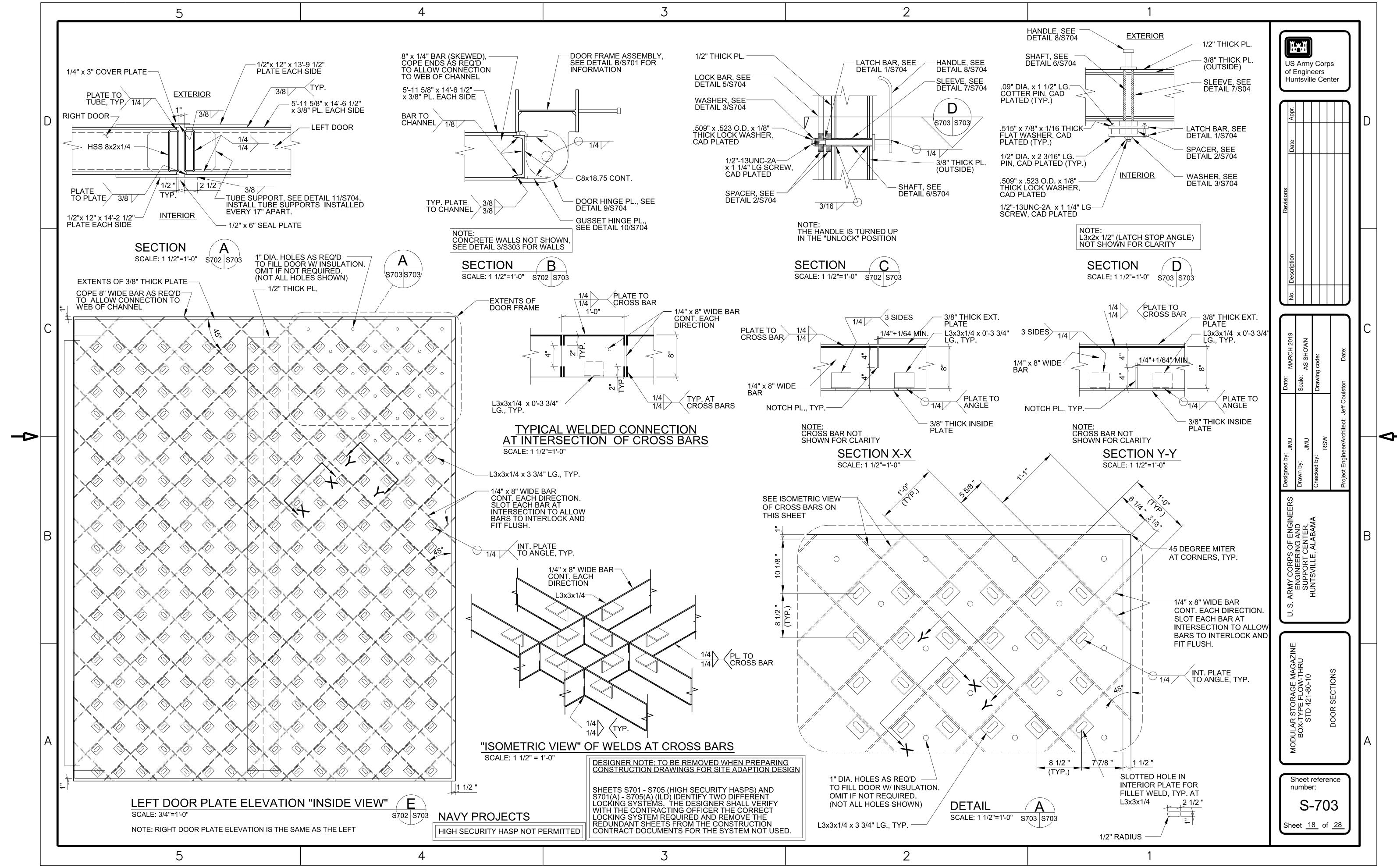


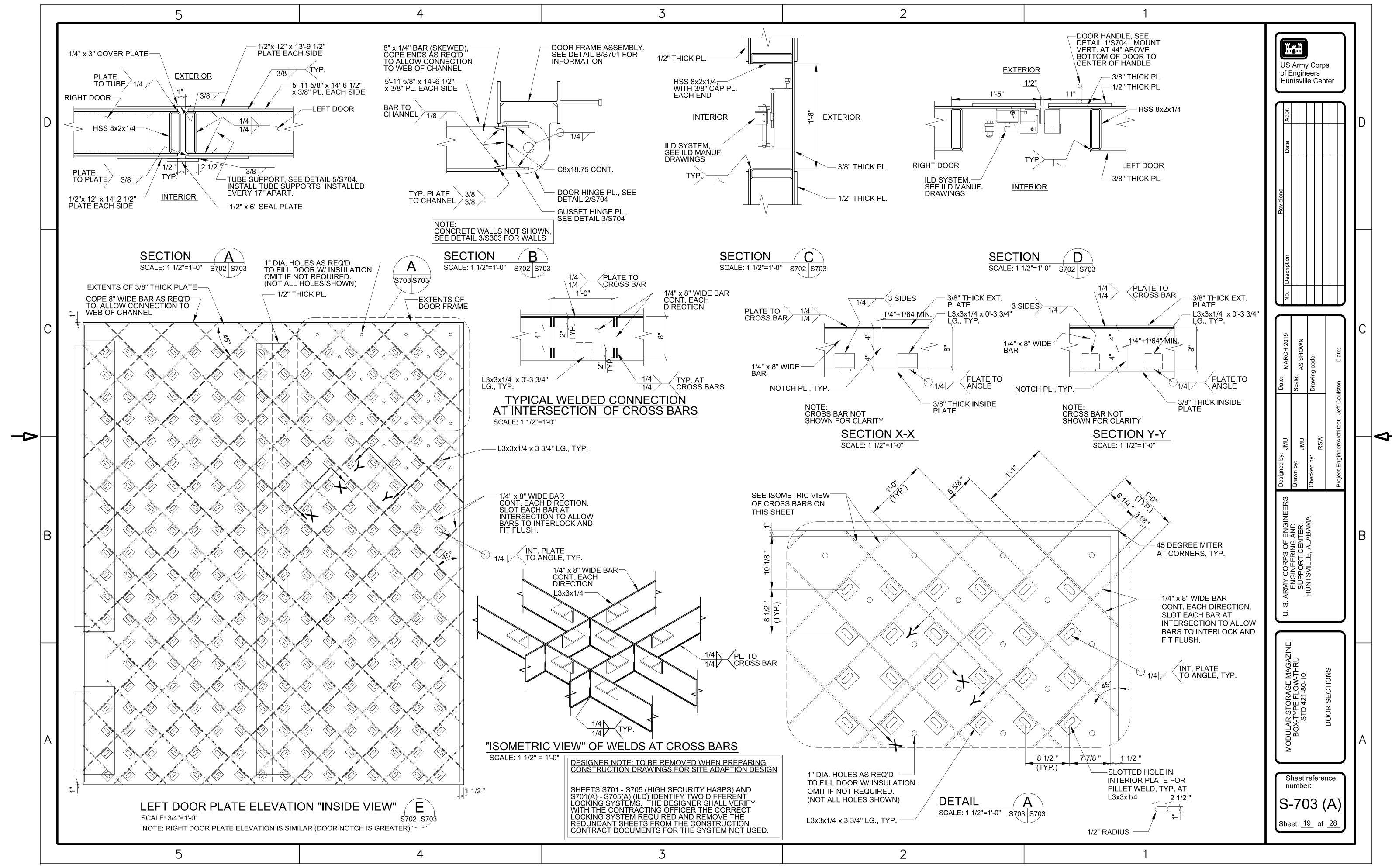
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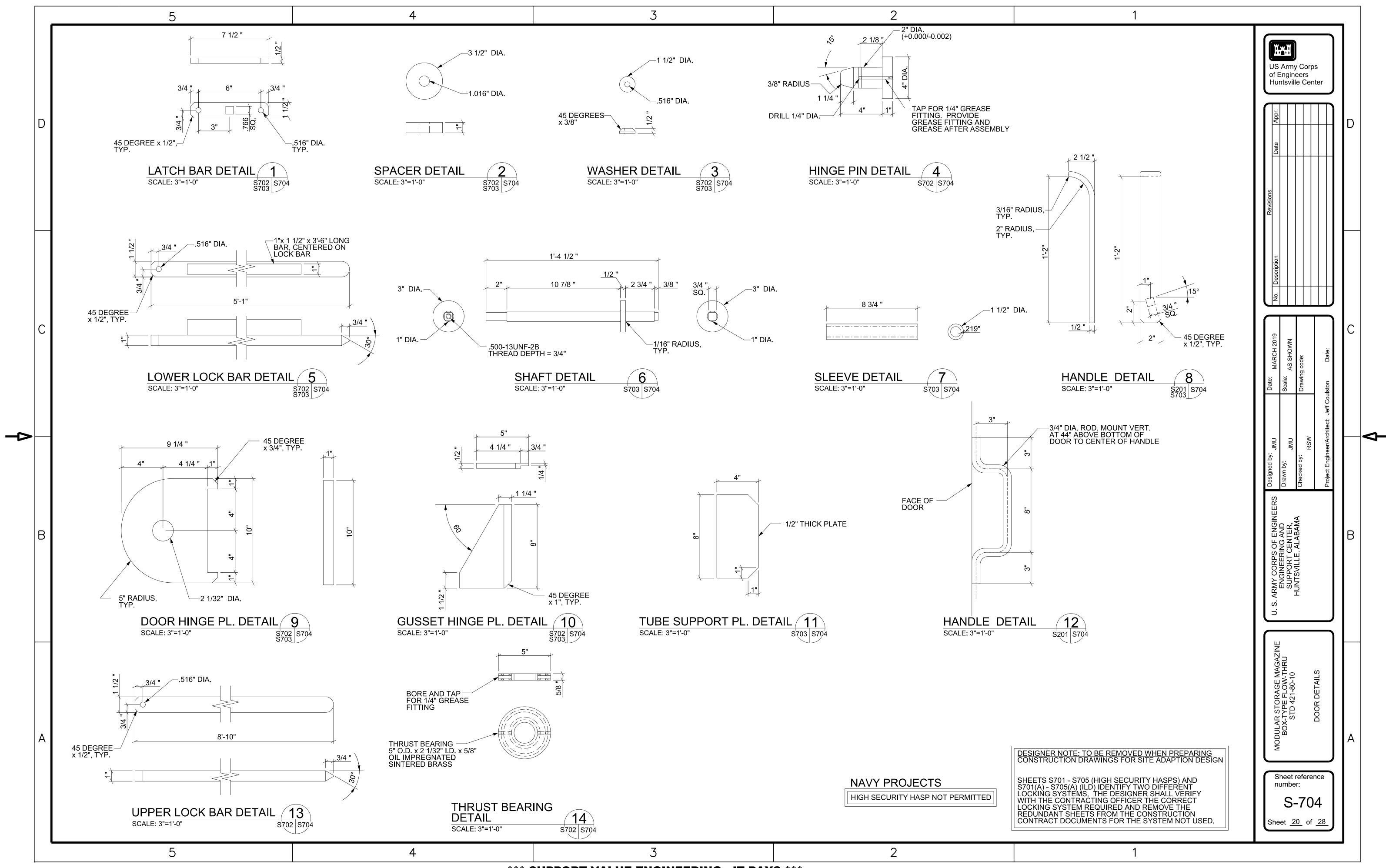
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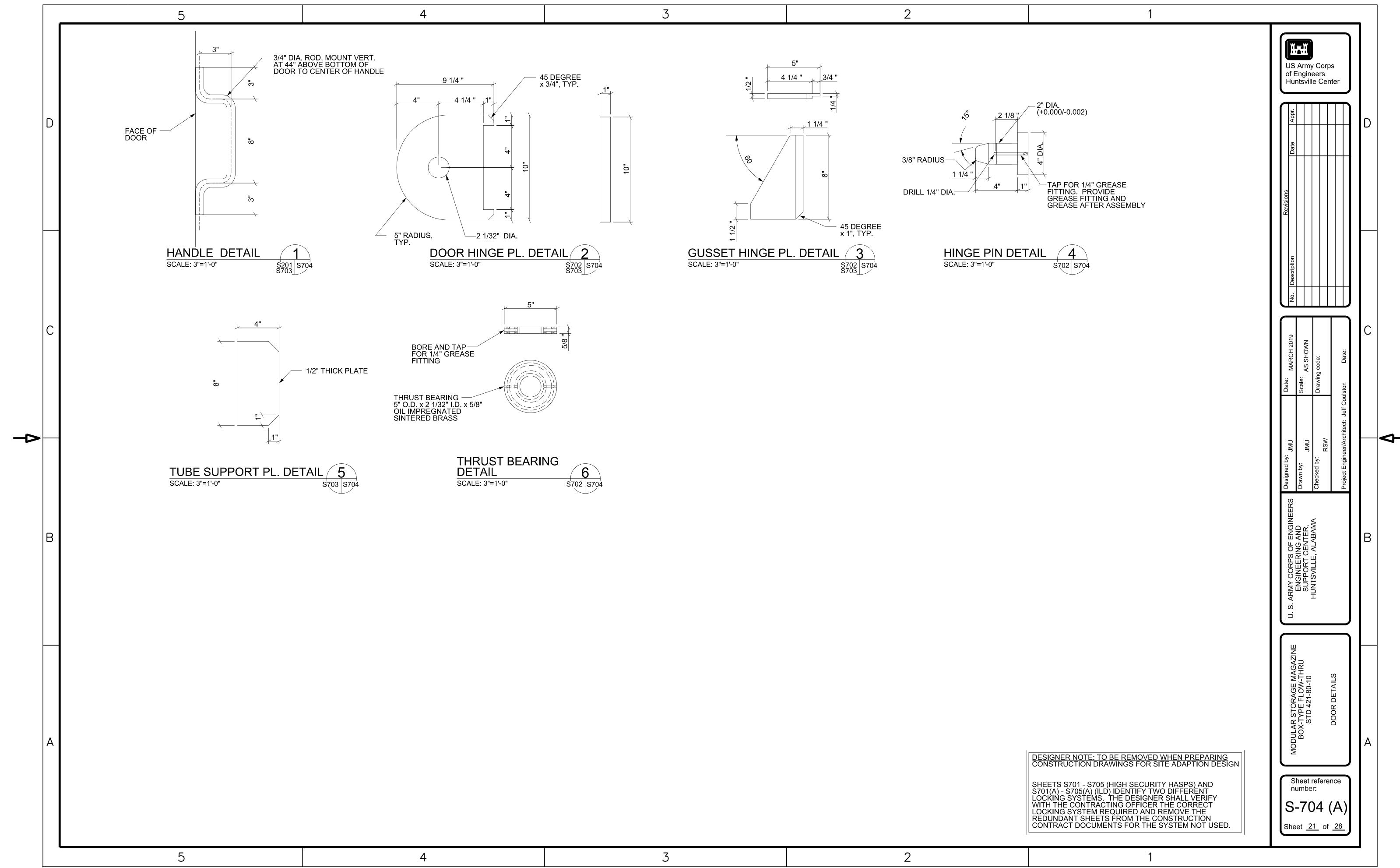


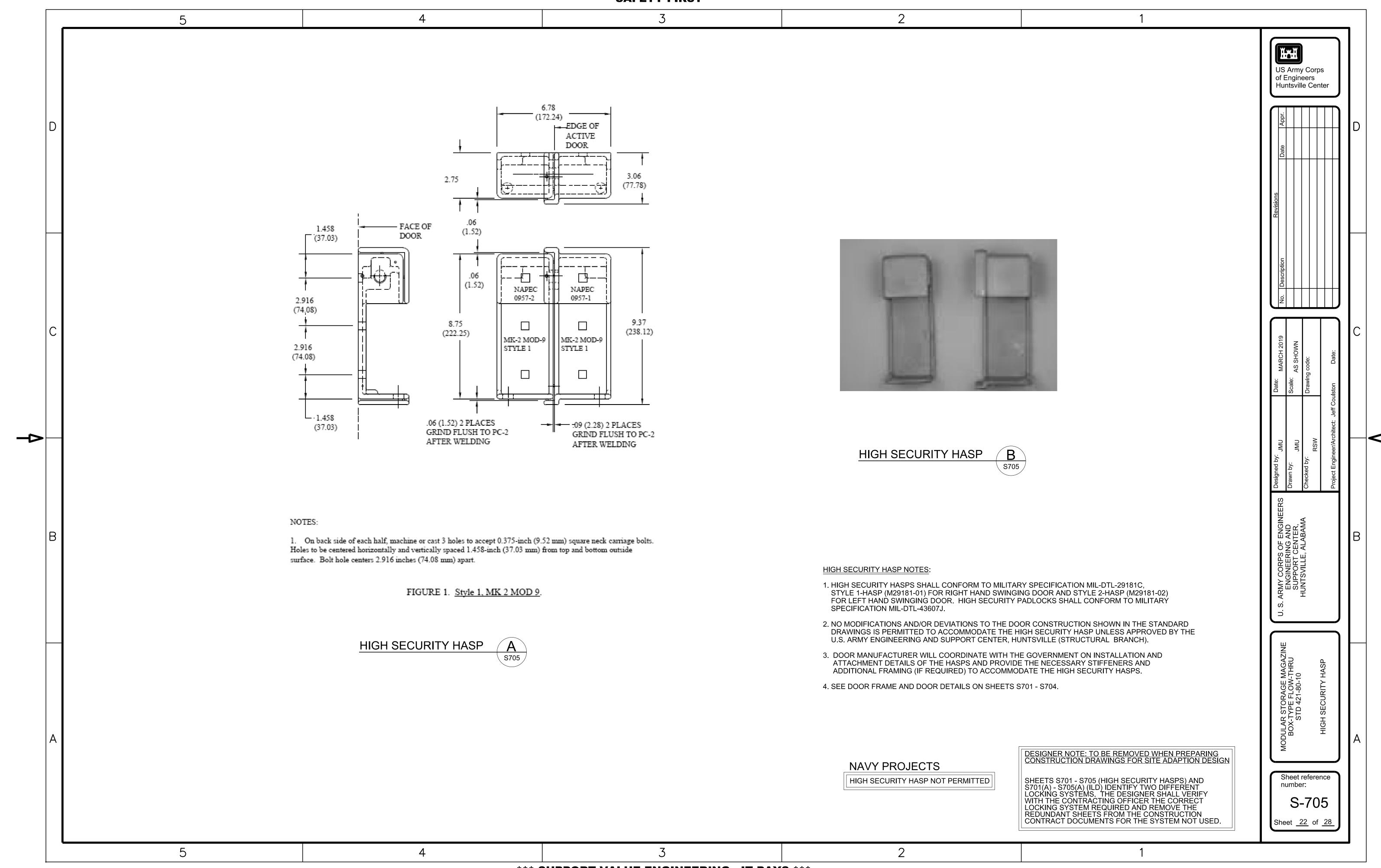


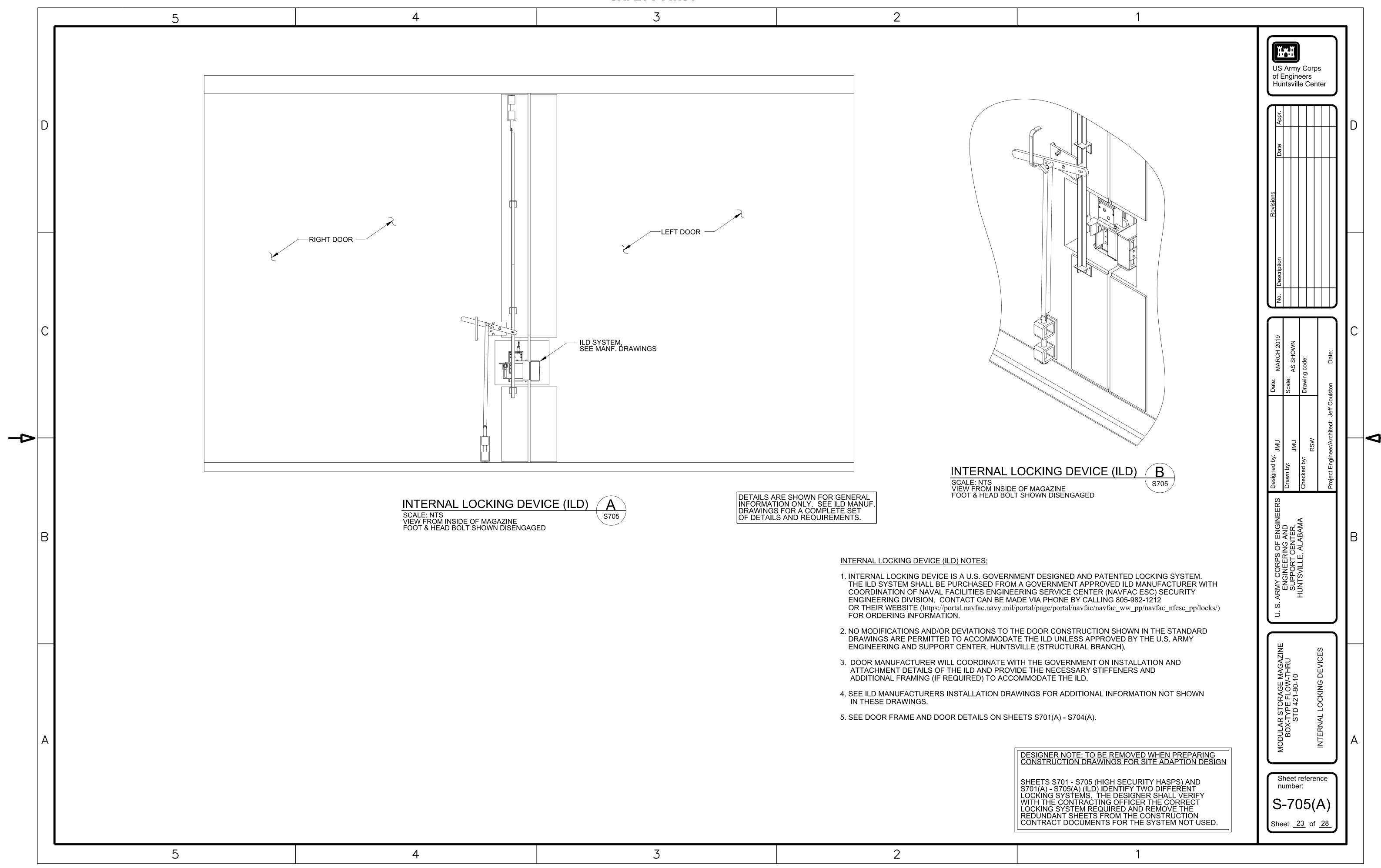


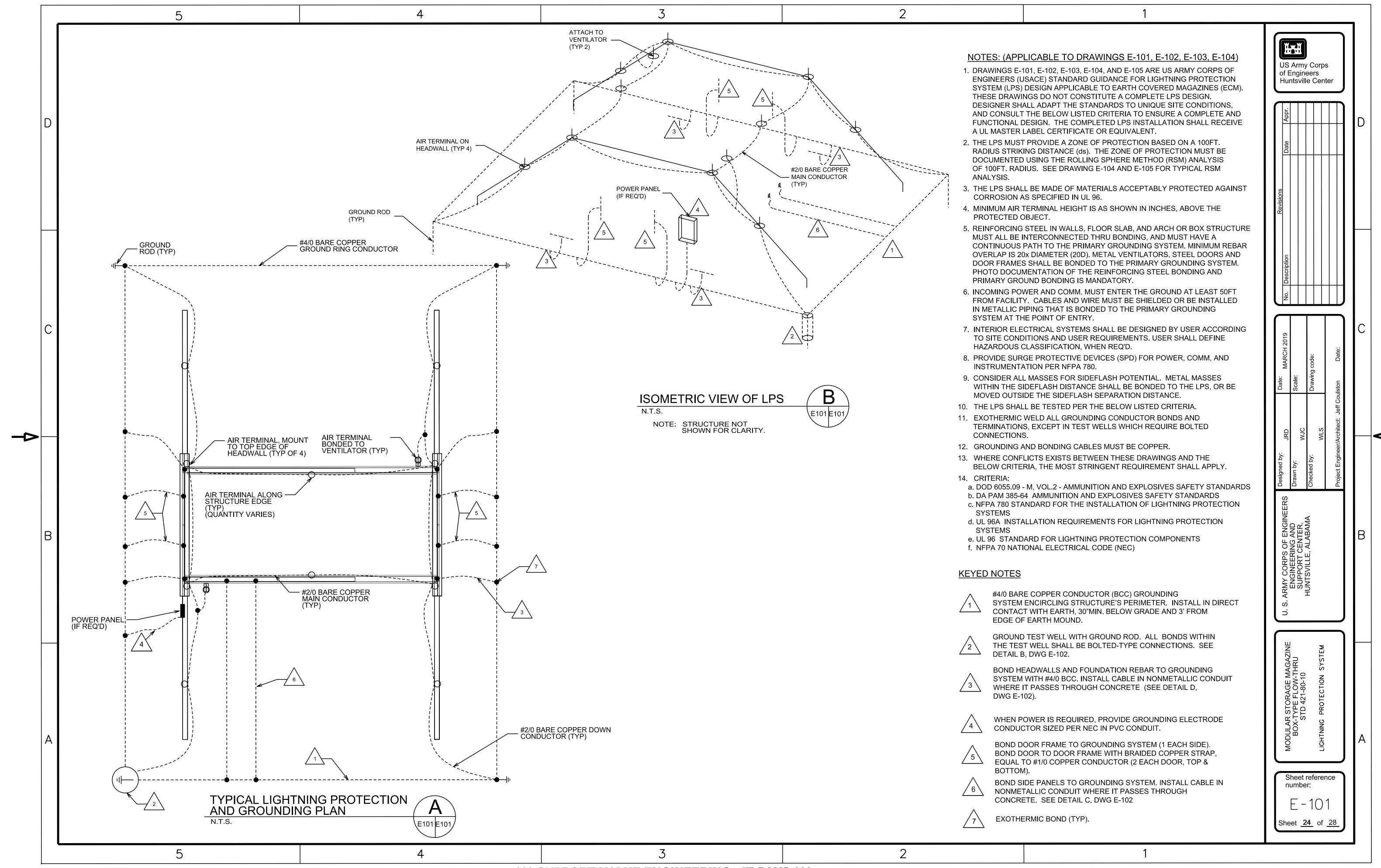
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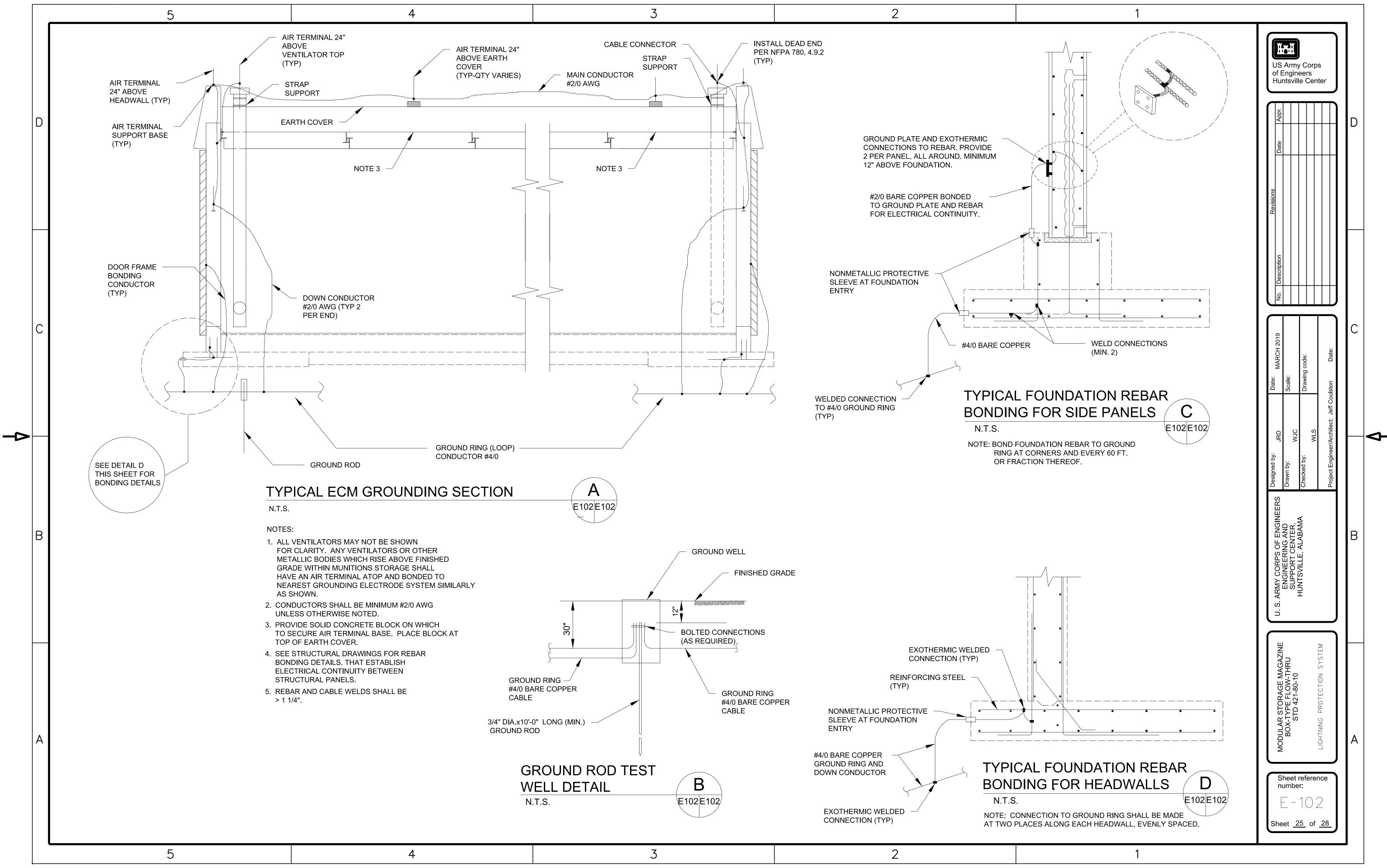












E-103

Sheet <u>26</u> of <u>28</u>